

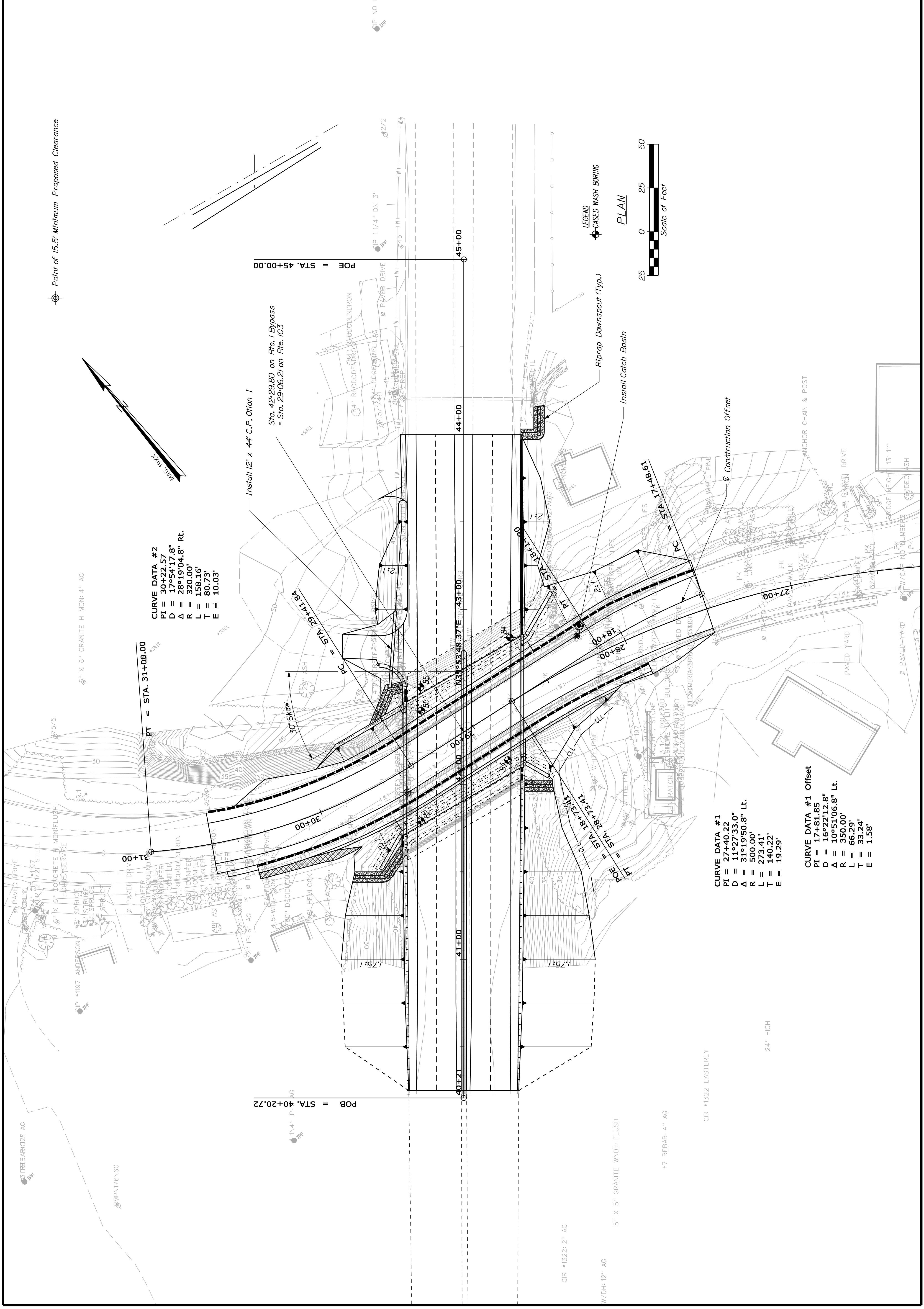
Addendum #1

To: File
Cc: TEDOCS
Author: Laura Krusinski
Subject: Soils Report 2008-20C
Addendum #1
Doc Type: 24
Date: December 17, 2009
Bridge #: 1362
Route: Rte 1 over Rte 103 (Eliot Road)
PIN: 15099.00
Town: Kittery

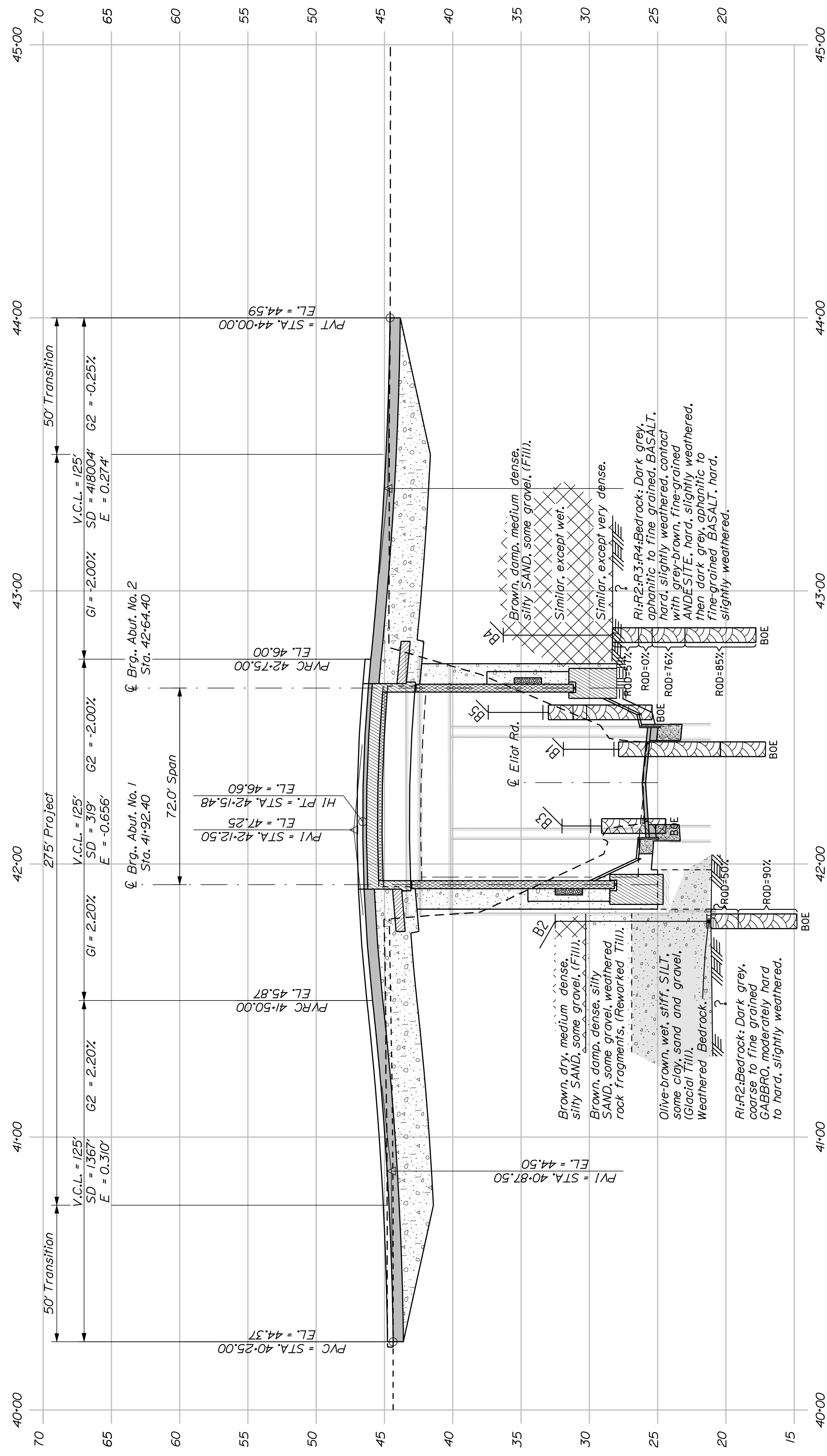
The following changes are made to the Geotechnical Design Report for the Replacement of Eliot Road Overpass carrying Route 1 over Route 103 (Eliot Road), Kittery, Maine, MaineDOT Soils Report No. 2008-20C (TEDOCS #933197).

1. Replace Figure 2 – Boring Location Plan and Interpretive Subsurface Profile, which has been updated with current station and elevation information, with the attached:
 - a. Sheet 1 – Boring Location Plan
 - b. Sheet 2 – Interpretive Subsurface Profile
 - c. Sheet 3 – Interpretive Subsurface Profile
2. Replace Appendix A – Boring Logs, with the attached Appendix A – Boring Logs, which has been updated with current station and elevation information.

PROJ. MANAGER	BY	DATE
DESIGN-DETAILED	L. KRUSINSKI	T. WHITE
CHECKED-REVIEWED		
DESIGNS-DETAILED		
REVISIONS 1		
REVISIONS 2		
REVISIONS 3		
REVISIONS 4		
FIELD CHANGES		



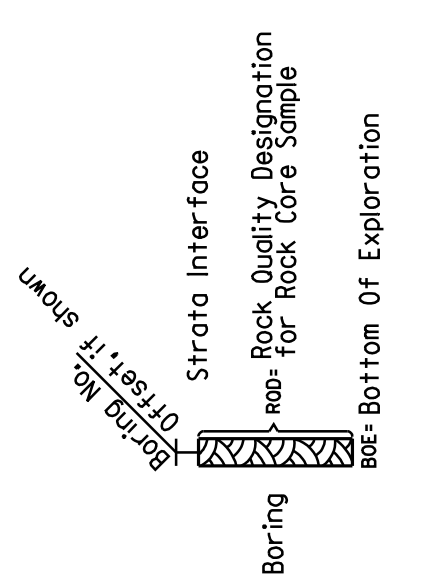
DATE	P.E. NUMBER	SIGNATURE



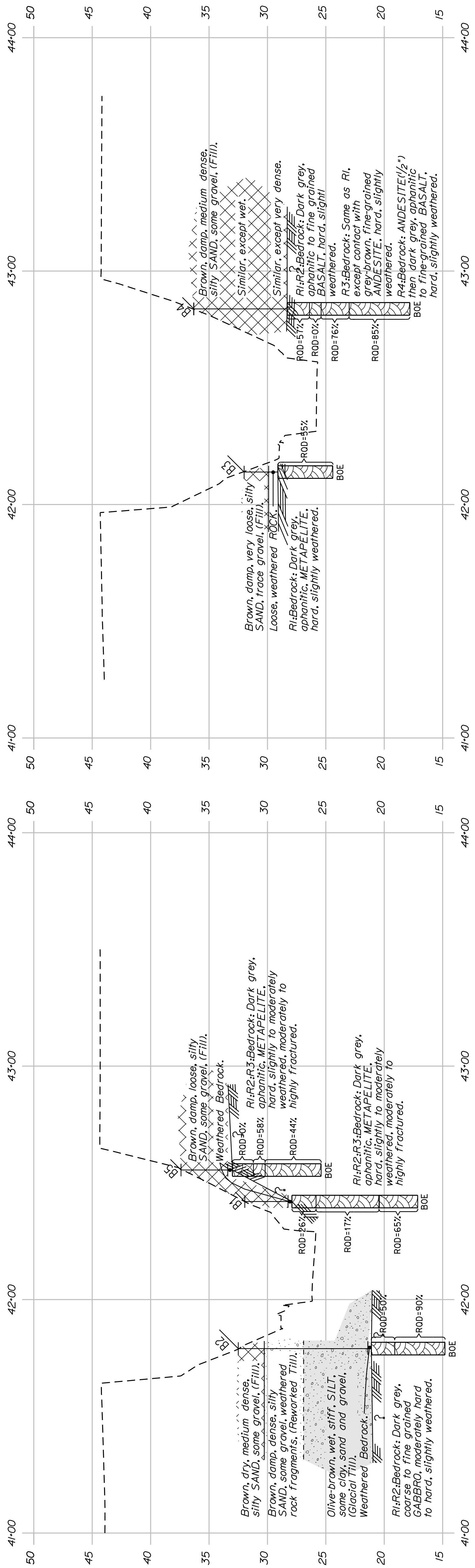
Notes: This generalized interpretive soil profile is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized, and have been developed by interpretations of widely spaced explorations and samples. Actual soil transitions may vary and are probably more erratic. For more specific information refer to the exploration logs.

Only details of Borings B2 and B4 shown.

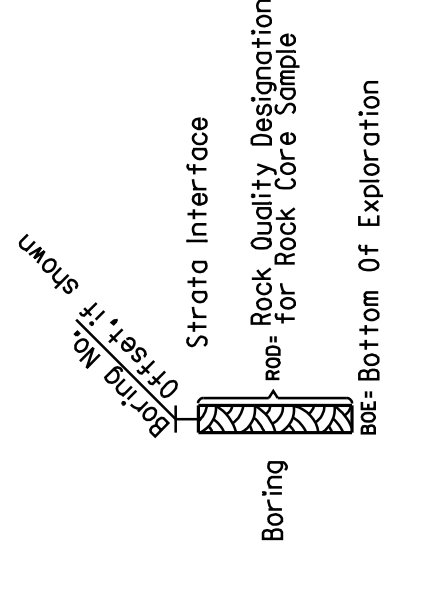
For Soil and Bedrock Stratigraphy, see Sheets 10 and 11, Interpretive Subsurface Profile and Boring Log Sheets.



DATE	P.E. NUMBER	SIGNATURE



Note: This generalized interpretive soil profile is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized, and have been developed by interpretations of widely spaced explorations and samples. Actual soil transitions may vary and are probably more erratic. For more specific information refer to the exploration logs.



Appendix A

Boring Logs

Driller: Maine Test Borings, Inc.	Elevation (ft.): 31.9	Auger ID/OD: N/A
Operator: D. McKeen	Datum: NAVD 88	Sampler: 24" Standard Split Spoon
Logged By: K. B. Stephenson	Rig Type: Mobile B53 Truck	Hammer Wt./Fall: 140#/30"
Date Start/Finish: 09/16/08	Drilling Method: Cased/Drive/Wash	Core Barrel: NQ-2"
Boring Location: 42+42, 24.4 Lt.	Casing ID/OD: NW	Water Level*: 8.95' bgs.

Definitions: D = Split Spoon Sample MD = Unsuccessful Split Spoon Sample attempt U = Thin Wall Tube Sample R = Rock Core Sample V = Insitu Vane Shear Test SSA = Solid Stem Auger	Definitions: S_u = Insitu Field Vane Shear Strength (psf) T_v = Pocket Torvane Shear Strength (psf) q_p = Unconfined Compressive Strength (ksf) $S_u(\text{lab})$ = Lab Vane Shear Strength (psf) WOH = weight of 140lb. hammer WOR = weight of rods	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test
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Depth (ft.)	Sample Information							Elevation (ft.)	Graphic Log	Visual Description and Remarks	Laboratory Testing Results/AASHTO and Unified Class.
	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows					
0	1D	24/8	0.0 - 2.0	2/6/4/4	10	3			Brown, damp, loose, silty fine to coarse SAND, some gravel, (FILL).		
						29					
						40					
						104					
5	R1	22.8/23	4.0 - 5.9	RQD = 26%		a39	28.20		WEATHERED BEDROCK. Drove NW casing to 3.7 ft. Advanced roller cone to 4.0 ft.	3.7	
						NQ-2 CORE	27.90		a39 blows for 0.2'	4.0	
	R2	64.8/64	6.1 - 11.5	RQD = 17%					R1: Dark gray, aphanitic, METAPELITE, hard, slightly to moderately weathered. Two low angle joints from 4.0 to 4.6 ft. Remainder of core is highly fractured, rusty, with clayey silt infill from 5.5 to 5.9 ft. R2: Dark grey, aphanitic, METAPELITE, hard, slightly to moderately weathered. Primary joint set is extremely close to close, horizontal to moderate, partly open to moderately wide, planar to undulating and rough. High angle secondary joint set present. Core is moderately fractured. Rusty discolorations on joint surfaces. Silt infilling from 6.1 to 7.1 ft.		
10											
	R3	39.6/40	11.5 - 14.8	RQD = 65%					R3: Same lithology as R1 and R2. Primary joint set same as R2 except extremely close to moderate. Rusty discolorations on joint surfaces.		
15							17.10		Bottom of Exploration at 14.80 feet below ground surface.	14.8	
20											
25											
30											

Remarks:
 Rope/cathead, safety hammer. 6 in. thinwall core of pavement

Driller: Maine Test Borings, Inc.	Elevation (ft.): 32.5	Auger ID/OD:
Operator: D. McKeen	Datum: NAVD 88	Sampler: 24" Standard Split Spoon
Logged By: K. B. Stephenson	Rig Type: B53 Mobile Truck	Hammer Wt./Fall: 140#/30"
Date Start/Finish: 09/16/08-09/17/08	Drilling Method: Cased/Drive/Wash	Core Barrel: NQ-2"
Boring Location: 41+79, 24.4 Lt.	Casing ID/OD: NW	Water Level*: 7.7' bgs.




Definitions: D = Split Spoon Sample MD = Unsuccessful Split Spoon Sample attempt U = Thin Wall Tube Sample R = Rock Core Sample V = Insitu Vane Shear Test SSA = Solid Stem Auger	Definitions: S _u = Insitu Field Vane Shear Strength (psf) T _v = Pocket Torvane Shear Strength (psf) q _p = Unconfined Compressive Strength (ksf) S _u (lab) = Lab Vane Shear Strength (psf) WOH = weight of 140lb. hammer WOR = weight of rods	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test
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Sample Information										Visual Description and Remarks	Laboratory Testing Results/AASHTO and Unified Class.
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log			
0	1D	24/4	0.0 - 2.0	2/4/14/18	18	3	30.30		Brown, dry, medium dense, silty coarse to fine SAND, some gravel, (FILL).		
						18					
						39					
	2D	24/14	3.0 - 5.0	16/20/15/11	35	14	21.40 21.10		Brown, damp, dense, silty coarse to fine SAND, some gravel, weathered rock fragments, (GLACIAL TILL DEPOSITS, possibly reworked)		
						22					
						15					
						7					
						6					
	3D	24/18	8.0 - 10.0	4/4/8/9	12	3	21.40 21.10		Olive-brown, wet, stiff, SILT, some clay and coarse to fine sand and gravel, bonded seams, (GLACIAL TILL DEPOSITS)		
						13					
						25					
	R1	24/18	11.4 - 13.4	RQD = 50%		a11	21.40 21.10		a11 blows for 0.1'. Advanced roller cone to 11.4 ft. (WEATHERED BEDROCK)		
	R2	51.6/54	13.4 - 17.7	RQD = 90%			14.80		R1: Dark grey, coarse to fine-grained. GABBRO, moderately hard to hard, slightly weathered. Primary joint set is very close to close, horizontal to low, open, planar to undulating and rough. R2: Same lithology as R1. Primary joint set same except extremely close to moderate.		
									Bottom of Exploration at 17.70 feet below ground surface.		

Remarks:
 Rope/cathead, safety hammer. 6 in. thinwall core of pavement

Driller: Maine Test Borings, Inc.	Elevation (ft.): 32.0	Auger ID/OD:
Operator: D. McKeen	Datum: NAVD 88	Sampler: 24" Standard Split Spoon
Logged By: K. B. Stephenson	Rig Type: B53 Mobile Truck	Hammer Wt./Fall: 140#/30"
Date Start/Finish: 09/17/08	Drilling Method: Cased/Drive/Wash	Core Barrel: NQ-2"
Boring Location: 42+13.9, 25.2 Rt.	Casing ID/OD: NW	Water Level*: 4.1' bgs.

Definitions: D = Split Spoon Sample MD = Unsuccessful Split Spoon Sample attempt U = Thin Wall Tube Sample R = Rock Core Sample V = Insitu Vane Shear Test SSA = Solid Stem Auger	Definitions: S_u = Insitu Field Vane Shear Strength (psf) T_v = Pocket Torvane Shear Strength (psf) q_p = Unconfined Compressive Strength (ksf) $S_u(\text{lab})$ = Lab Vane Shear Strength (psf) WOH = weight of 140lb. hammer WOR = weight of rods	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test
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Depth (ft.)	Sample Information							Elevation (ft.)	Graphic Log	Visual Description and Remarks	Laboratory Testing Results/AASHTO and Unified Class.
	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows	WOH				
0	1D	24/8	0.0 - 2.0	1/1/1/6	2	WOH			Brown, damp, very loose, brown silty fine SAND, trace coarse to medium sand and gravel, (FILL).		
						3	29.90				
	2D R1	7/7 56.4/54	2.1 - 2.7 2.9 - 7.6	44/50(1.2") RQD = 55%		11 a50	29.10		Loose, gray weathered rock fragments, some silty SAND (WEATHERED BEDROCK)		
5									R1: Dark grey, aphanitic, METAPELITE, hard, slightly weathered. Primary joint set is extremely close to moderate, horizontal to moderate, open, planar to undulating and rough. High angle secondary joints at 3.8 and 7.0 ft. Calcite/quartz stringers present throughout. Rusty discolorations on joints. a50 blows for 0.6'.		
							24.40		Bottom of Exploration at 7.60 feet below ground surface. Note: piece of rock from core barrel blocks borehole- unable to core beyond 7.6 ft.		
10											
15											
20											
25											
30											

Remarks:
Rope/cathead, safety hammer. 6 in. thinwall core of pavement

Maine Department of Transportation Soil/Rock Exploration Log US CUSTOMARY UNITS	Project: Elliot Road Overpass over Route 103	Boring No.: B4
	Location: Kittery, Maine	PIN: 15099.00

Driller: Maine Test Borings, Inc.	Elevation (ft.): 36.3	Auger ID/OD:
Operator: D. McKeen	Datum:	Sampler: 24" Standard Split Spoon
Logged By: K. B. Stephenson	Rig Type: B53 Mobile Truck	Hammer Wt./Fall: 140#/30"
Date Start/Finish: 09/17/08-09/18/08	Drilling Method: Cased/Drive/Wash	Core Barrel: NQ-2"
Boring Location: 42+83.8, 26.3 Rt.	Casing ID/OD: NW	Water Level*: 11.7' bgs.

Definitions: D = Split Spoon Sample MD = Unsuccessful Split Spoon Sample attempt U = Thin Wall Tube Sample R = Rock Core Sample V = Insitu Vane Shear Test SSA = Solid Stem Auger	Definitions: S_u = Insitu Field Vane Shear Strength (psf) T_v = Pocket Torvane Shear Strength (psf) q_p = Unconfined Compressive Strength (ksf) $S_u(\text{lab})$ = Lab Vane Shear Strength (psf) WOH = weight of 140lb. hammer WOR = weight of rods	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test
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Depth (ft.)	Sample Information						Elevation (ft.)	Graphic Log	Visual Description and Remarks	Laboratory Testing Results/AASHTO and Unified Class.
	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows				
0	1D	24/10	0.0 - 2.0	3/5/7/10	12	5	28.30		Brown, damp, medium dense, silty coarse to fine SAND, some gravel, (FILL). Brown, wet, medium dense, silty coarse to fine SAND, some gravel, (FILL). a30 blows for 0.9'. Brown, wet, very dense, silty coarse to fine SAND, some gravel. (FILL) Split spoon refusal at 7.9 ft. Advanced roller cone to 8.0 ft. R1: Dark grey, aphanitic to fine-grained, BASALT, hard, slightly weathered. Primary joint set is extremely close to close, horizontal to moderate, open, planar to undulating and rough. Rusty discolorations on joint surfaces. R2: Same lithology and joint set as R1. R3: Same lithology and joint set as R1, except primary joint set is very close to moderate, horizontal to low angle. At 12.9 ft., contact with grey-brown fine-grained ANDESITE, hard, slightly weathered. R4: Approximately 1/2 in. end of contact with ANDESITE. Dark grey, aphanitic to fine grained BASALT, hard, slightly weathered. Fine-grained ANDESITE intrusions at 14.4, 15.9 and 18.3 ft. Host rock inclusions and secondary mineralization from 17.7 to 18.0 ft.	
						14				
	2D	24/8	2.5 - 4.5	13/14/8/11	22	10				
						16				
5						21				
						13				
						16				
	3D	9.6/7	7.1 - 7.9	12/50(3.6")		a30				
	R1	22.8/13	8.0 - 9.9	RQD = 57%		NO-2 CORE				
10	R2	12/12	9.9 - 10.9	RQD = 0%						
	R3	28.8/27	10.9 - 13.3	RQD = 76%						
	R4	61.2/62	13.3 - 18.4	RQD = 85%						
15										
20										
25										
30										

Remarks:
Rope/cathead, safety hammer. 6 in. thinwall core of pavement

Maine Department of Transportation Soil/Rock Exploration Log US CUSTOMARY UNITS	Project: Elliot Road Overpass over Route 103 Location: Kittery, Maine	Boring No.: B5 PIN: 15099.00
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Driller: Maine Test Borings, Inc.	Elevation (ft.): 37.4	Auger ID/OD:
Operator: D. McKeen	Datum: NAVD 88	Sampler: 24" Standard Split Spoon
Logged By: K. B. Stephenson	Rig Type: B53 Mobile Truck	Hammer Wt./Fall: 140#/30"
Date Start/Finish: 09/18/08	Drilling Method: Cased/Spin/Wash	Core Barrel: NQ-2"
Boring Location: 42+55.5, 24.8 Lt.	Casing ID/OD: NW	Water Level*: 19.7' below top of pavement

Definitions: D = Split Spoon Sample MD = Unsuccessful Split Spoon Sample attempt U = Thin Wall Tube Sample R = Rock Core Sample V = Insitu Vane Shear Test SSA = Solid Stem Auger	Definitions: S_u = Insitu Field Vane Shear Strength (psf) T_v = Pocket Torvane Shear Strength (psf) q_p = Unconfined Compressive Strength (ksf) $S_u(\text{lab})$ = Lab Vane Shear Strength (psf) WOH = weight of 140lb. hammer WOR = weight of rods	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test
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Depth (ft.)	Sample Information								Elevation (ft.)	Graphic Log	Visual Description and Remarks	Laboratory Testing Results/AASHTO and Unified Class.
	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows						
0										Brown, damp, loose, silty fine to coarse SAND, some gravel (FILL).		
5	R1	21.6/24	4.4 - 6.2	RQD = 0%		NQ-2 CORE		33.40 33.00		Spun NW casing to 4.3 ft. Roller bit through weathered bedrock from 4.0 to 4.4 ft.		
	R2	12/15	6.2 - 7.2	RQD = 58%						R1: Dark grey, aphanitic, METAPELITE, hard, slightly to moderately weathered. Primary joint set is extremely close to close, horizontal to moderate, open, planar to undulating and rough. High angle secondary joints present throughout. Core is moderately to highly fractured. Rusty discolorations on joints; many vugs, pits. R2: Same lithology as R1. One horizontal joint and one high angle joint present. Joints are open, close, planar to undulating and rough. Some vugs, rusty discolorations. R3: Same lithology as R1 and R2. Primary joints same as R1, except extremely close to moderate. High angle secondary joint present to 10.7 ft. Rusty discolorations on joints. Solid core from 10.7 to 12.0 ft.		
	R3	57.6/57	7.2 - 12.0	RQD = 44%								
10								25.40				
15												
20												
25												
30												

Remarks:
Rope/cathead, safety hammer. 6 in. thinwall core of pavement

UNIFIED SOIL CLASSIFICATION SYSTEM				TERMS DESCRIBING DENSITY/CONSISTENCY																							
MAJOR DIVISIONS		GROUP SYMBOLS		TYPICAL NAMES																							
COARSE-GRAINED SOILS (more than half of material is larger than No. 200 sieve size)	GRAVELS (more than half of coarse fraction is larger than No. 4 sieve size)	CLEAN GRAVELS	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	<p>Coarse-grained soils (more than half of material is larger than No. 200 sieve): Includes (1) clean gravels; (2) silty or clayey gravels; and (3) silty, clayey or gravelly sands. Consistency is rated according to standard penetration resistance.</p> <p style="text-align: center;">Modified Burmister System</p> <table border="0"> <tr> <td style="text-align: center;"><u>Descriptive Term</u></td> <td style="text-align: center;"><u>Portion of Total</u></td> </tr> <tr> <td>trace</td> <td>0% - 10%</td> </tr> <tr> <td>little</td> <td>11% - 20%</td> </tr> <tr> <td>some</td> <td>21% - 35%</td> </tr> <tr> <td>adjective (e.g. sandy, clayey)</td> <td>36% - 50%</td> </tr> </table> <table border="0"> <tr> <td style="text-align: center;"><u>Density of Cohesionless Soils</u></td> <td style="text-align: center;"><u>Standard Penetration Resistance</u> <u>N-Value (blows per foot)</u></td> </tr> <tr> <td>Very loose</td> <td>0 - 4</td> </tr> <tr> <td>Loose</td> <td>5 - 10</td> </tr> <tr> <td>Medium Dense</td> <td>11 - 30</td> </tr> <tr> <td>Dense</td> <td>31 - 50</td> </tr> <tr> <td>Very Dense</td> <td>> 50</td> </tr> </table>	<u>Descriptive Term</u>	<u>Portion of Total</u>	trace	0% - 10%	little	11% - 20%	some	21% - 35%	adjective (e.g. sandy, clayey)	36% - 50%	<u>Density of Cohesionless Soils</u>	<u>Standard Penetration Resistance</u> <u>N-Value (blows per foot)</u>	Very loose	0 - 4	Loose	5 - 10	Medium Dense	11 - 30	Dense	31 - 50	Very Dense	> 50
		<u>Descriptive Term</u>	<u>Portion of Total</u>																								
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Loose	5 - 10																										
Medium Dense	11 - 30																										
Dense	31 - 50																										
Very Dense	> 50																										
(little or no fines)	GP	Poorly-graded gravels, gravel sand mixtures, little or no fines																									
GRAVEL WITH FINES (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures.																									
	GC	Clayey gravels, gravel-sand-clay mixtures.																									
SANDS (more than half of coarse fraction is smaller than No. 4 sieve size)	CLEAN SANDS (little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines																								
		SP	Poorly-graded sands, gravelly sand, little or no fines.																								
	SANDS WITH FINES (Appreciable amount of fines)	SM	Silty sands, sand-silt mixtures																								
		SC	Clayey sands, sand-clay mixtures.																								
FINE-GRAINED SOILS (more than half of material is smaller than No. 200 sieve size)	SILTS AND CLAYS (liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.																								
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.																								
		OL	Organic silts and organic silty clays of low plasticity.																								
	SILTS AND CLAYS (liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.																								
		CH	Inorganic clays of high plasticity, fat clays.																								
		OH	Organic clays of medium to high plasticity, organic silts																								
HIGHLY ORGANIC SOILS	Pt	Peat and other highly organic soils.																									
<p>Desired Soil Observations: (in this order)</p> <p>Color (Munsell color chart) Moisture (dry, damp, moist, wet, saturated) Density/Consistency (from above right hand side) Name (sand, silty sand, clay, etc., including portions - trace, little, etc.) Gradation (well-graded, poorly-graded, uniform, etc.) Plasticity (non-plastic, slightly plastic, moderately plastic, highly plastic) Structure (layering, fractures, cracks, etc.) Bonding (well, moderately, loosely, etc., if applicable) Cementation (weak, moderate, or strong, if applicable, ASTM D 2488) Geologic Origin (till, marine clay, alluvium, etc.) Unified Soil Classification Designation Groundwater level</p>				<p>Rock Quality Designation (RQD):</p> <p>RQD = $\frac{\text{sum of the lengths of intact pieces of core}^* > 100 \text{ mm}}{\text{length of core advance}}$</p> <p>*Minimum NQ rock core (1.88 in. OD of core)</p> <p style="text-align: center;">Correlation of RQD to Rock Mass Quality</p> <table border="0"> <tr> <td style="text-align: center;"><u>Rock Mass Quality</u></td> <td style="text-align: center;"><u>RQD</u></td> </tr> <tr> <td>Very Poor</td> <td><25%</td> </tr> <tr> <td>Poor</td> <td>26% - 50%</td> </tr> <tr> <td>Fair</td> <td>51% - 75%</td> </tr> <tr> <td>Good</td> <td>76% - 90%</td> </tr> <tr> <td>Excellent</td> <td>91% - 100%</td> </tr> </table> <p>Desired Rock Observations: (in this order)</p> <p>Color (Munsell color chart) Texture (aphanitic, fine-grained, etc.) Lithology (igneous, sedimentary, metamorphic, etc.) Hardness (very hard, hard, mod. hard, etc.) Weathering (fresh, very slight, slight, moderate, mod. severe, severe, etc.) Geologic discontinuities/jointing: -dip (horiz - 0-5, low angle - 5-35, mod. dipping - 35-55, steep - 55-85, vertical - 85-90) -spacing (very close - <5 cm, close - 5-30 cm, mod. close 30-100 cm, wide - 1-3 m, very wide >3 m) -tightness (tight, open or healed) -infilling (grain size, color, etc.) Formation (Waterville, Ellsworth, Cape Elizabeth, etc.) RQD and correlation to rock mass quality (very poor, poor, etc.) ref: AASHTO Standard Specification for Highway Bridges 17th Ed. Table 4.4.8.1.2A Recovery</p>		<u>Rock Mass Quality</u>	<u>RQD</u>	Very Poor	<25%	Poor	26% - 50%	Fair	51% - 75%	Good	76% - 90%	Excellent	91% - 100%										
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<p>Maine Department of Transportation Geotechnical Section Key to Soil and Rock Descriptions and Terms Field Identification Information</p>				<p>Sample Container Labeling Requirements:</p> <table border="0"> <tr> <td>PIN</td> <td>Blow Counts</td> </tr> <tr> <td>Bridge Name / Town</td> <td>Sample Recovery</td> </tr> <tr> <td>Boring Number</td> <td>Date</td> </tr> <tr> <td>Sample Number</td> <td>Personnel Initials</td> </tr> <tr> <td>Sample Depth</td> <td></td> </tr> </table>		PIN	Blow Counts	Bridge Name / Town	Sample Recovery	Boring Number	Date	Sample Number	Personnel Initials	Sample Depth													
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