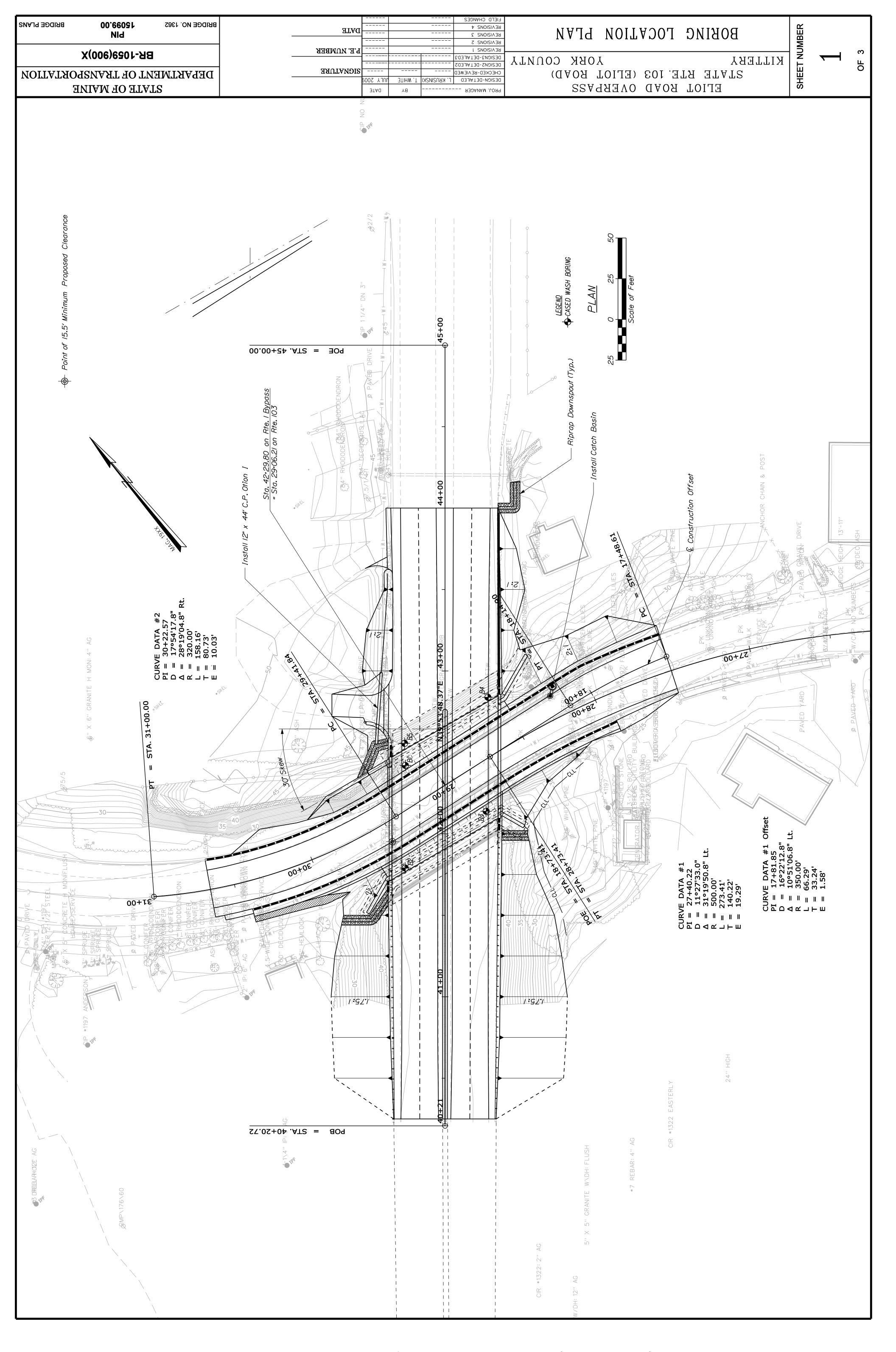
Addendum #1

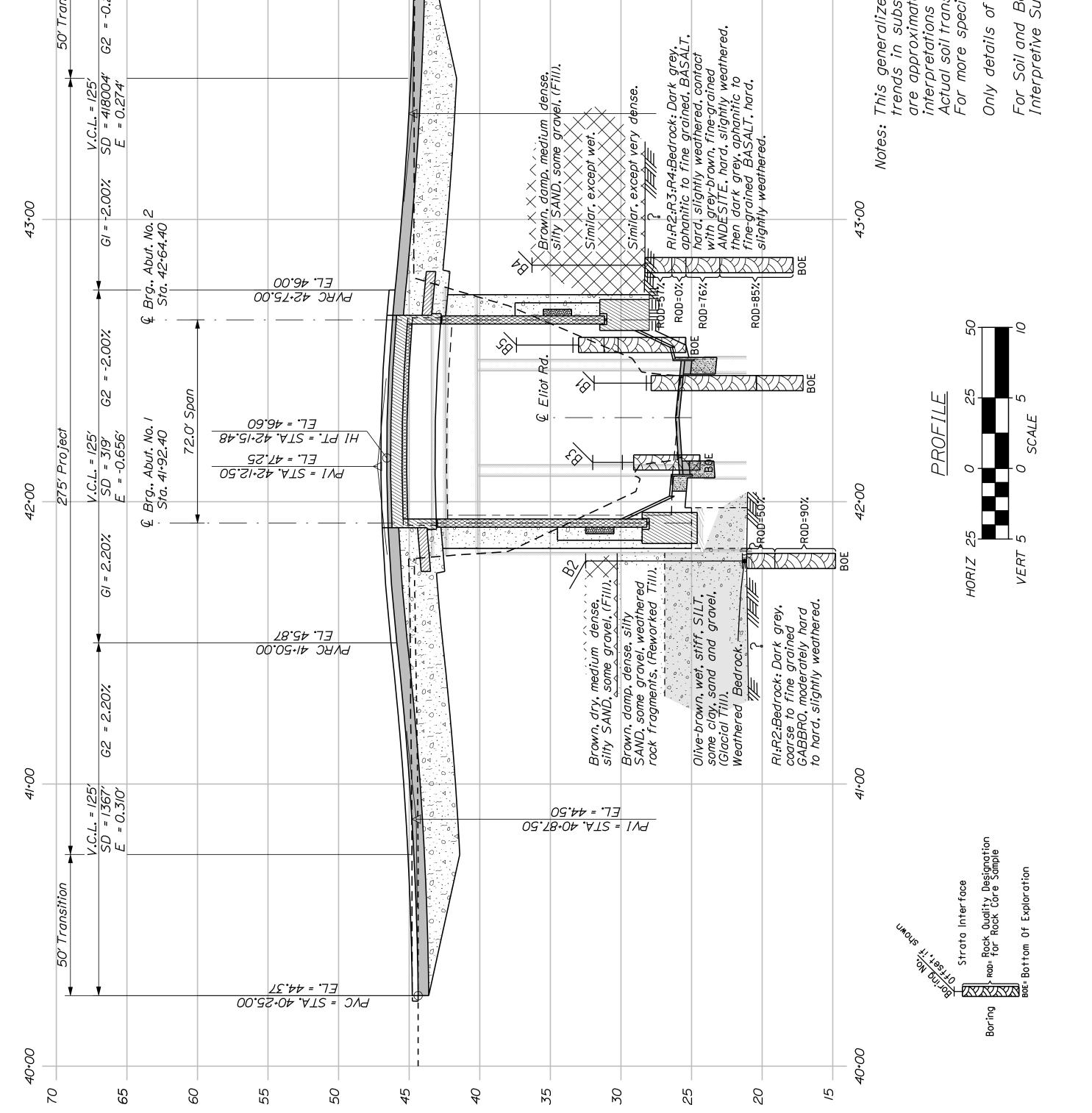
To:	File
Cc:	TEDOCS
Author:	Laura Krusinski
Subject:	Soils Report 2008-20C
	Addendum #1
Doc Type:	24
Date:	December 17, 2009
Bridge #:	1362
Route:	Rte 1 over Rte 103 (Eliot Road)
PIN:	15099.00
Town:	Kittery

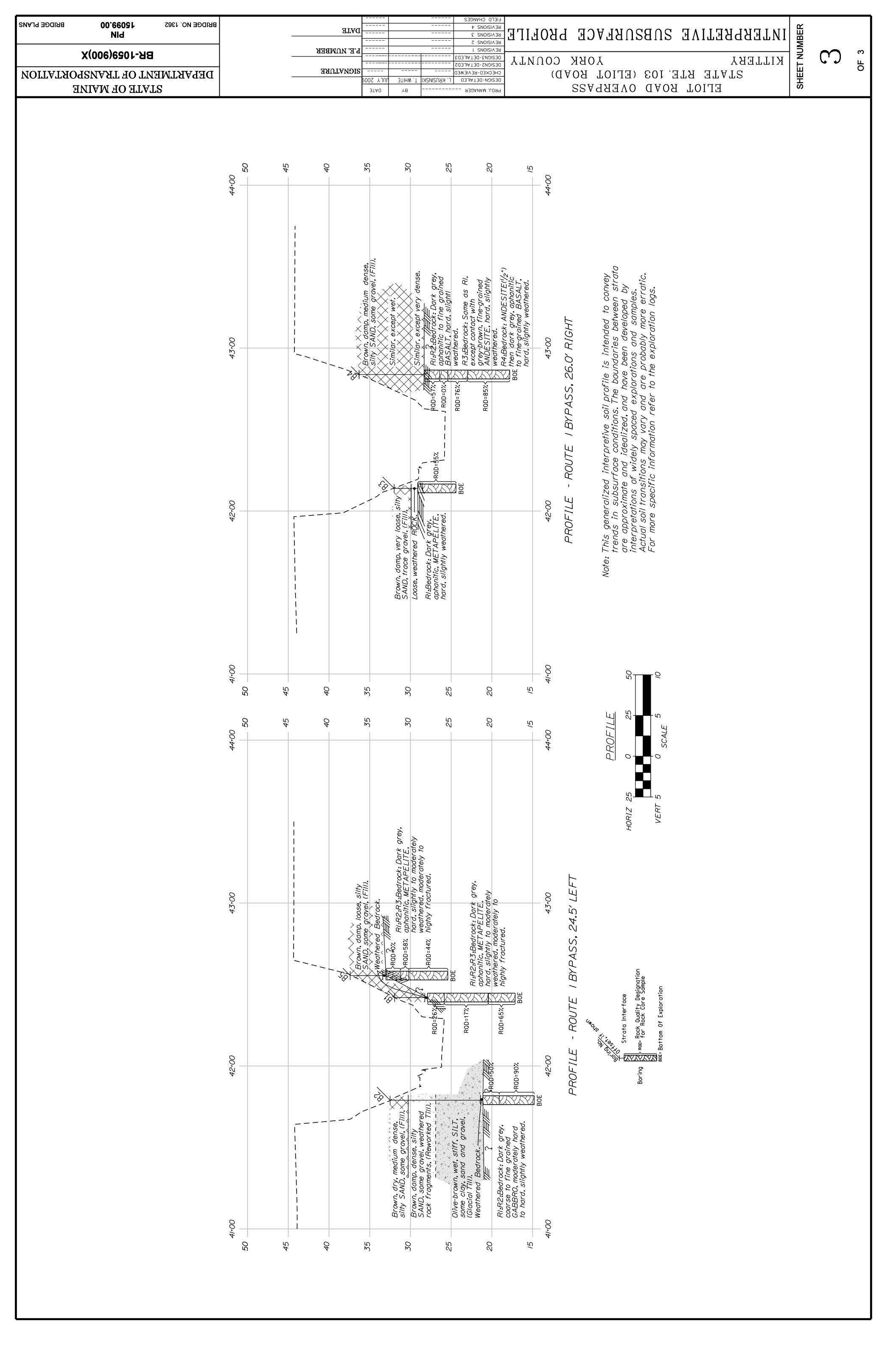
The following changes are made to the Geotechnical Design Report for the Replacement of Eliot Road Overpass carrying Route 1 over Route 103 (Eliot Road), Kittery, Maine, MaineDOT Soils Report No. 2008-20C (TEDOCS #933197).

- 1. Replace Figure 2 Boring Location Plan and Interpretive Subsurface Profile, which has been updated with current station and elevation information, with the attached:
 - a. Sheet 1 Boring Location Plan
 - b. Sheet 2 Interpretive Subsurface Profile
 - c. Sheet 3 Interpretive Subsurface Profile
- 2. Replace Appendix A Boring Logs, with the attached Appendix A Boring Logs, which has been updated with current station and elevation information.



BRIDGE PLANS	00 [.] 66031 NIG	GE NO. 1362	פאונ			I			ELEED CHANCES BEAISIONS 4 BEAISIONS 2	-	ЪВО	ACE	IAUSAUR	IAE 3	ТЭЯЧЯЭ	LNI	BER		
	Х(006)6501-ЯЯ				S'E' NOMBEK				KEAISIONS 5 BEAISIONS 1 DESICN3-DELHIFED3	-						TINT	SHEET NUMBER	\sim	ന
NULLUN	NT OF TRANSPO		Π		BIGNATURE				DESIGNS-DETAILED2	_		ХОВК ОЛ КО	103 (EFI	जि.स.भ	TERY Strate	цТЛ	EET		OF
	TATE OF MAINE					DATE UULY 2009		L, KRUSINSK	РROJ. МАИАСЕР DESIGN-DETAILED	-			OVD OVE				SHI		
	00-34 1	22		60 L	<u>5</u> 20		40	35	30	25			45+00 ted to convey between strata veloped by	samples. more erratic. ation logs.	and II. Sheets.				
	0044	Transition = -0.25%		00°C	<u>69°++ = 73</u> 00+++ YIS = INJ								44.00 hized interpretive soil profile is intend ubsurface conditions. The boundaries u mate and idealized, and have been de	widely spaced exp ons may vary and information refer	sorings BZ and B4 snown. drock Statigraphy, see Sheets 10 surface Profile and Boring Log				





Appendix A

Boring Logs

I	Maine	e Depa	rtment	of Transporta	tion	F	Project:	Elliot I	Road Overpass over Route 103	Boring No.:	E	81
		<u>S</u>	oil/Rock Expl JS CUSTOMA	loration Log			ocation	1: Kitte	ery, Maine	PIN:	1509	99.00
Drille	r:]	Maine Test Bo	orings. Inc.	Ele	vation ((ft.)	31.9		Auger ID/OD:	N/A	
Oper			D. McKeen	6.7		um:			7D 88	Sampler:	24" Standard S	plit Spoon
<u> </u>	ed By:]	K. B. Stephens	son	Rig	Type:		Mob	ile B53 Truck	Hammer Wt./Fall:	140#/30"	1
			09/16/08		<u> </u>	ling Me	thod:		d/Drive/Wash	Core Barrel:	NQ-2"	
Borir	g Locat		42+42, 24.4 Lt	t.	-	ing ID/		NW		Water Level*:	8.95' bgs.	
MD = U U = Th R = Ro V = Ins	lit Spoon S	ul Split Spoor e Sample ample hear Test	n Sample attemp	x	S _u = T _v = q _p = S _{u(la} WOH	Pocket To Unconfination (ab) = Lab	Vane Shea t of 140lb.	ear Stren essive Str ar Strengt	gth (psf) ength (ksf)	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test		
		1	1 1	Sample Information		i		1				Laboratory
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log	Visual Descri	ption and Remarks		Testing Results/ AASHTO and Unified Class.
0	1D	24/8	0.0 - 2.0	2/6/4/4	10	3			Brown, damp, loose, silty fine to co	arse SAND, some gravel, (I	FILL).	
						29						
						40	_					
						104	28.20 27.90		\ \WEATHERED BEDROCK. Drove	NW casing to 3.7 ft Adva		
- 5 -	R1	22.8/23	4.0 - 5.9	RQD = 26%		a39 −NQ-2-	4	96	to 4.0 ft.	itte casing to 5.7 ft. Adva	4.0	
						CORE	_		a39 blows for 0.2'. R1: Dark gray, aphanitic, METAPE	LITE hard slightly to mod		
	R2	64.8/64	6.1 - 11.5	RQD = 17%			-	90	weathered. Two low angle joints fro	m 4.0 to 4.6 ft. Remainder		
							-		fractured, rusty, with clayey silt infi R2: Dark grey, aphanitic, METAPE	LITE, hard, slightly to mod		
							-		weathered. Primary joint set is extre partly open to moderately wide, pla	nar to undulating and rough	n. High angle	
- 10 -							-		secondary joint set present. Core is n on joint surfaces. Silt infilling from		discolorations	
	R3	39.6/40	11.5 - 14.8	RQD = 65%				916) 916)	R3: Same lithology as R1 and R2. P		except extremely	
							_		close to moderate. Rusty discolorati	ons on joint surfaces.		
						\mathbb{H}	-					
- 15 -						¥.	17.10	1784117	Bottom of Exploration at	14.80 feet below ground s	14.8- 114.8-	
]					
							4					
- 20 -							-					
							-					
							-					
							-					
							-					
- 25 -							-					
							-					
							-					
							1					
20							1					
<u>30</u> <u>Rema</u>	arks:	1	<u> </u>	I		I		1				
Rop	e/cathead,	, safety han	nmer. 6 in. thi	nwall core of pavement								

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.	Page 1 of 1
* Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.	Boring No.: B1

Ι	Maine	e Depa	rtment	of Transporta	ition	F	Project:	Elliot	Road Overpass over Route 103	Boring No.:	E	32
		<u>S</u>	oil/Rock Expl IS CUSTOMA	oration Log			ocatior	n: Kitt	ery, Maine	PIN:	1509	99.00
Drille	r:		Maine Test Bo	rings, Inc.	Elev	/ation ((ft.)	32.5		Auger ID/OD:		
Opera	ator:		D. McKeen		Dati	um:		NA	/D 88	Sampler:	24" Standard Sp	plit Spoon
Logg	ed By:		K. B. Stephens	on	Rig	Туре:		B53	Mobile Truck	Hammer Wt./Fall:	140#/30"	
Date	Start/Fir	nish:	09/16/08-09/17	7/08	Drill	ling Me	thod:	Case	d/Drive/Wash	Core Barrel:	NQ-2"	
Borin	g Locat	ion:	41+79, 24.4 Lt		Cas	ing ID/	OD:	NW		Water Level*:	7.7' bgs.	
MD = U U = Thi R = Ro V = Ins	it Spoon S	ul Split Spoo e Sample mple near Test	n Sample attemp	t	S _u = T _v = q _p = S _{u(la} WOH	Pocket To Unconfine b) = Lab I = weight	ons: Definitions: situ Field Vane Shear Strength (psf) WC = water content, percent ocket Torvane Shear Strength (psf) LL = Liquid Limit confined Compressive Strength (ksf) PL = Plastic Limit = Lab Vane Shear Strength (psf) PI = Plasticity Index : weight of 140lb. hammer G = Grain Size Analysis : weight of rods C = Consolidation Test					
				Sample Information								Laboratory
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psf) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log		ption and Remarks		Testing Results/ AASHTO and Unified Class.
0	1D	24/4	0.0 - 2.0	2/4/14/18	18	3	-		Brown, dry, medium dense, silty co	arse to fine SAND, some g	ravel, (FILL).	
						39	30.30				2.2-	
	2D	24/14	3.0 - 5.0	16/20/15/11	35	14			Brown, damp, dense, silty coarse to fragments, (GLACIAL TILL DEPO		weathered rock	
_						22			0			
- 5 -						15	1					
						7	1					
						6	1					
	3D	24/18	8.0 - 10.0	4/4/8/9	12	3			Olive-brown, wet, stiff, SILT, some bonded seams, (GLACIAL TILL D		d and gravel,	
- 10 -						13	4					
	R1	24/18	11.4 - 13.4	RQD = 50%		25 a11	21.40		a11 blows for 0.1'.			
		2.710					- 21.10		Advanced roller cone to 11.4 ft. (W	EATHERED BEDROCK)	11.1-	
	R2	51.6/54	13.4 - 17.7	RQD = 90%			-	1.24.14	R1: Dark grey, coarse to fine-graine			
- 15 -	K2	51.0/54	13.4 - 17.7	KQD = 7070				+ 10-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	slightly weathered. Primary joint set open, planar to undulating and rougl R2: Same lithology as R1. Primary moderate.	h.		
							-	55				
							14.80	14. 	Bottom of Exploration at	17.70 feet below ground s	urface.	
- 20 -							-					
							-					
							-					
- 25 -							1					
							-					
							-					
30 Rema	ırks:											
		safety han	nmer. 6 in. thir	nwall core of pavement								

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.	Page 1 of 1
* Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.	Boring No.: B2

N	Aaine	e Depa	artment	of Transporta	tion	F	Project:	Elliot	Road Overpass over Route 103	Boring No.:	E	33
			Soil/Rock Expl	-		L	_ocation	: Kitte	ery, Maine	PIN:	1509	99.00
Drille	r:		Maine Test Bo	rings. Inc.	Elev	ation ((ft.)	32.0		Auger ID/OD:		
Opera	ator:		D. McKeen	0,	Datu		. ,	NAV	/D 88	Sampler:	24" Standard S	plit Spoon
Logge	ed By:		K. B. Stephens	on	Rig	Туре:		B53	Mobile Truck	Hammer Wt./Fall:	140#/30"	
Date \$	Start/Fin	ish:	09/17/08		Drill	ing Me	thod:	Case	ed/Drive/Wash	Core Barrel:	NQ-2"	
Borin	g Locati	ion:	42+13.9, 25.2 1	Rt.	Casi	ng ID/	OD:	NW		Water Level*:	4.1' bgs.	
MD = U U = Thir R = Roc V = Insi	it Spoon Sa	ul Split Spoo e Sample mple near Test	n Sample attemp		T _V = F q _p = U S _{u(lal} WOH	nsitu Fie Pocket To Jnconfino () = Lab	Vane Shea t of 140lb. I	ear Stren ssive Str ar Streng	gth (psf) ength (ksf)	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test		
			1	Sample Information			1					Laboratory
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (pst) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log	Visual Descr	iption and Remarks		Testing Results/ AASHTO and Unified Class.
0	1D	24/8	0.0 - 2.0	1/1/1/6	2	WOH			Brown, damp, very loose, brown sil and gravel, (FILL).	ty fine SAND, trace coarse	to medium sand	
						3	29.90				2.1-	
	2D 	7/7 - 56.4/54	2.1 - 2.7 2.9 - 7.6	44/50(1.2") RQD = 55%		11 a50	29.10	21152	Loose, gray weathered rock fragmer (WEATHERED BEDROCK)	nts, some silty SAND		
- 5 - 10 - 15 - 20							24.40		R1: Dark grey, aphanitic, METAPE set is extremely close to moderate, undulating and rough. High angle so quartz stringers present throughou. 1 350 blows for 0.6'. Bottom of Exploration at Note: piece of rock from core barren ft.	horizontal to moderate, ope econdary joints at 3.8 and 7 Rusty discolorations on join 7.60 feet below ground su	en, planar to .0 ft. Calcite/ nts. 7.6- urface.	
30 <u>Rema</u> Rope		safety har	nmer. 6 in. thi	nwall core of pavement								

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.	Page 1 of 1
* Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.	Boring No.: B3

]	Maine	e Depa	artment	of Transporta	tion	F	Project:	Elliot	Road Overpass over Route 103	Boring No.:	E	34
		<u> </u>	Soil/Rock Explo JS CUSTOMA	oration Log			-		ery, Maine	PIN:	1509	99.00
Drille					Ele		£4.)	26.2		Auger ID/OD:		
	ator:		Maine Test Bor D. McKeen	nings, nic.		vation (um:)	36.3		Sampler:	24" Standard S	nlit Spoon
<u> </u>	ed By:		K. B. Stephens	on.		Type:		D52	Mobile Truck	Hammer Wt./Fall:	140#/30"	pin spoon
	Start/Fir	vieh:	09/17/08-09/18		<u> </u>	ling Me	thod:		ed/Drive/Wash	Core Barrel:	NQ-2"	
	ng Locat		42+83.8, 26.3 H			sing ID/		NW		Water Level*:	11.7' bgs.	
Definit	ions:		42+05.0, 20.51	κι.	Defir	nitions:				Definitions:	11.7 Ugs.	
MD = 0 U = Th R = Ro V = Ins	bilt Spoon S Jnsuccessf in Wall Tub ock Core Sa situ Vane Sl Solid Stem	ul Split Spoo e Sample ample hear Test	on Sample attempt	Sample Information	T _V = q _p = S _{u(la} WOH	Pocket To	orvane Sh ed Compre Vane She of 140lb.	ear Strer essive St ar Streng	rength (ksf)	WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test	t 	I
												Laboratory
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psť) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log		iption and Remarks		Testing Results/ AASHTO and Unified Class.
0	1D	24/10	0.0 - 2.0	3/5/7/10	12	5			Brown, damp, medium dense, silty	coarse to fine SAND, some	e gravel, (FILL).	
						14						
	2D	24/8	2.5 - 4.5	13/14/8/11	22	10	1		Brown, wet, medium dense, silty co	arse to fine SAND, some	gravel, (FILL).	
						16			,,,,,,,,.,.,,.,,.,,.			
						21	1					
- 5 -						13	1					
						16	-					
	3D	9.6/7	7.1 - 7.9	12/50(3.6")		a30	-		^a 30 blows for 0.9'.			
	R1	22.8/13	8.0 - 9.9	RQD = 57%		NO-2	28.30)))))))	Brown, wet, very dense, silty coarse \spoon refusal at 7.9 ft. Advanced ro		el. (FILL) Split	
	R1 R2	12/12	9.9 - 10.9	RQD = 37% $RQD = 0%$		CORE	-		R1: Dark grey, aphanitic to fine-gra	ined, BASALT, hard, sligh		
- 10 -				-			-	<u>ISN</u>	Primary joint set is extremely close to undulating and rough. Rusty disc			t
	R3	28.8/27	10.9 - 13.3	RQD = 76%			-		R2: Same lithology and joint set as R3: Same lithology and joint set as	R1.		
							-		moderate, horizontal to low angle. A	At 12.9 ft., contact with gre		
							-	au	grained ANDESITE, hard, slightly			
	R4	61.2/62	13.3 - 18.4	RQD = 85%			-		R4: Approximately 1/2 in. end of co to fine grained BASALT, hard, slight			
- 15 ·							-		intrusions at 14.4, 15.9 and 18.3 ft. mineralization from 17.7 to 18.0 ft.	Host rock inclusions and se	econdary	
							1					
						$\sqrt{7}$	1					
						$\vdash \forall$	17.80					
							- 17.00	, 	Bottom of Exploration at	18.50 feet below ground s		
- 20 ·							-					
							-					
							-					
							-					
							-					
- 25 -							4					
							-					
							4					
							1					
30												
Rem	arks:											
Rop	e/cathead,	, safety har	nmer. 6 in. thir	wall core of pavement								
I												

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.	Page 1 of 1
* Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.	Boring No.: B4

I	Maine	e Depa	artment (of Transporta	tion	Р	roject:	Elliot l	Road Overpass over Route 103	Boring No.:	E	35
		-	Soil/Rock Explo JS CUSTOMA	Q		L	ocation	: Kitte	ery, Maine	PIN:	1509	99.00
Drille	r:		Maine Test Bor	rings, Inc.	Eleva	ation (ft.)	37.4		Auger ID/OD:		
Oper	ator:		D. McKeen		Datu	m:		NAV	/D 88	Sampler:	24" Standard Sp	plit Spoon
Logg	ed By:		K. B. Stephenso	on	Rig 1	Гуре:		B53	Mobile Truck	Hammer Wt./Fall:	140#/30"	
Date	Start/Fir	nish:	09/18/08		Drilli	ing Me	thod:	Case	ed/Spin/Wash	Core Barrel:	NQ-2"	
	g Locat	ion:	42+55.5, 24.8 I	Lt.	_	ng ID/0	DD:	NW		Water Level*:	19.7' below top	of pavement
$ \begin{array}{llllllllllllllllllllllllllllllllllll$					nsitu Fiel Pocket To Jnconfine) = Lab \ = weight	rvane She d Compre /ane Shea of 140lb. ł	ar Stren ssive Str r Strengt	gth (psf) ength (ksf)	Definitions: WC = water content, percent LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index G = Grain Size Analysis C = Consolidation Test			
				Sample Information			1					Laboratory
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (pst) or RQD (%)	N-value	Casing Blows	Elevation (ft.)	Graphic Log	Visual Descri	ption and Remarks		Testing Results/ AASHTO and Unified Class.
0							-		Brown, damp, loose, silty fine to co	arse SAND, some gravel (F	ILL).	
- 5 -	R1	21.6/24	4.4 - 6.2	RQD = 0%		NQ-2 CORE	33.40 33.00		Spun NW casing to 4.3 ft. Roller bi	-	4.4-	
	R2	12/15	6.2 - 7.2	RQD = 58%			1	9990	R1: Dark grey, aphanitic, METAPE weathered. Primary joint set is extre			
	R3	57.6/57	7.2 - 12.0	RQD = 44%				999	open, planar to undulating and roug	h. High angle secondary joi	nts present	
- 10 - - 15 - - 20 -				KQD = 44%			25.40		throughout. Core is moderately to h joints; many vugs, pits. R2: Same lithology as R1. One hor Joints are open, close, planar to und discolorations. R3: Same lithology as R1 and R2. P close to moderate. High angle secon discolorations on joints. Solid core f Bottom of Exploration at	izontal joint and one high a lulating and rough. Some v rimary joints same as R1, e dary joint present to 10.7 fi from 10.7 to 12.0 ft.	ngle joint present. 1gs, rusty xcept extremely . Rusty 12.0-	
<u>30</u> <u>Rema</u>	arks:								<u> </u>			
Rope	e/cathead	, safety haı	nmer. 6 in. thir	nwall core of pavement								

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.	Page 1 of 1
* Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.	Boring No.: B5

UNIFIED SOIL CLASSIFICATION SYSTEM					TERMS DESCRIBING DENSITY/CONSISTENCY			
MA.I	OR DIVISIO		GROUP SYMBOLS	TYPICAL NAMES				
COARSE- GRAINED	GRAVELS	CLEAN GRAVELS	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	<u>Coarse-grained soils</u> (more than half of material is larger than No. 200 sieve): Includes (1) clean gravels; (2) silty or clayey gravels; and (3) silty, clayey or gravelly sands. Consistency is rated according to standard			
(more than half of material is larger than No. 200 sieve size) S	(more than half of coarse fraction is larger than No. 4 sieve size)	(little or no fines)	GP	Poorly-graded gravels, gravel sand mixtures, little or no fines	<u>Descript</u> tra	penetration resistance. Modified Burmister System <u>Descriptive Term</u> trace 0% - 10%		
		GRAVEL WITH FINES	GM	Silty gravels, gravel-sand-sill mixtures.			11% - 20% 21% - 35% 36% - 50%	
	(mc fract	(Appreciable amount of fines)	GC	Clayey gravels, gravel-sand-clay mixtures.				rd Penetration Resistance Value (blows per foot) 0 - 4 5 - 10
	SANDS	CLEAN SANDS	SW	Well-graded sands, gravelly sands, little or no fines	Medium Dense31 - 30Dense31 - 50Very Dense> 50		11 - 30 31 - 50	
	(more than half of coarse fraction is smaller than No. 4 sieve size)	(little or no fines)	SP	Poorly-graded sands, gravelly sand, little or no fines.	Fine-grained soils (more than half of material is smaller than No. 20(sieve): Includes (1) inorganic and organic silts and clays; (2) gravelly, sand, or silty clays; and (3) clayey silts. Consistency is rated according to shear strength as indicated. <u>Approximate</u>			
		SANDS WITH FINES	SM	Silty sands, sand-silt mixtures				
		(Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures.	Consistency of Cohesive soils	<u>SPT N-Value</u> blows per foot WOH, WOR,	<u>Undrained</u> <u>Shear</u> Strength (psf)	<u>Field</u> Guidelines
(more than half of material is S B H Smaller than No. 200 sieve size) S H H C H H	SILTS AND CLAYS (liquid limit less than 50)		ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.	Very Soft Soft Medium Stiff Stiff	WOP, <2 2 - 4 5 - 8 9 - 15	0 - 250 250 - 500 500 - 1000 1000 - 2000	Fist easily Penetrates Thumb easily penetrates Thumb penetrates with moderate effort Indented by thumb with
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.	Very Stiff Hard	16 - 30 >30	2000 - 2000 2000 - 4000 over 4000	Indented by thumb with great effort Indented by thumbnai Indented by thumbnail with difficulty
			OL	Organic silts and organic silty clays of low plasticity.	Rock Quality Designation (RQD): RQD = sum of the lengths of intact pieces of core* > 100 mm length of core advance *Minimum NQ rock core (1.88 in. OD of core)			
	SILTS AND CLAYS		ΜΗ	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.	Correlation of RQD to Rock Mass Quality Rock Mass Quality RQD			
			СН	Inorganic clays of high plasticity, fat clays.	Very Poor Poor Fair Good		<25% 26% - 50% 51% - 75% 76% - 90%	
			OH	Organic clays of medium to high plasticity, organic silts	Excellent 91% - 100% Desired Rock Observations: (in this order) Color (Munsell color chart) Texture (aphanitic, fine-grained, etc.)			
	SO	ORGANIC	Pt	Peat and other highly organic soils.	Lithology (igneous, sedimentary, metamorphic, etc.) Hardness (very hard, hard, mod. hard, etc.) Weathering (fresh, very slight, slight, moderate, mod. severe,			
Desired So			is order)			severe, etc.)		
Color (Munsell color chart) Moisture (dry, damp, moist, wet, saturated)					Geologic discon	tinuities/jointing: -dip (horiz - 0-5, lo	w angle - 5-35 m	nod. dippina -
Density/Consistency (from above right hand side) Name (sand, silty sand, clay, etc., including portions - trace, little, etc.) Gradation (well-graded, poorly-graded, uniform, etc.) Plasticity (non-plastic, slightly plastic, moderately plastic, highly plastic) Structure (layering, fractures, cracks, etc.) Bonding (well, moderately, loosely, etc., if applicable) Cementation (weak, moderate, or strong, if applicable, ASTM D 2488)							o - 55-85, vertical	
						-spacing (very close - <5 cm, close - 5-30 cm, mod. close 30-100 cm, wide - 1-3 m, very wide >3 m) -tightness (tight, open or healed) -infilling (grain size, color, etc.) Formation (Waterville, Ellsworth, Cape Elizabeth, etc.) POD and correlation to rock mass quality (very poor, etc.)		
Geologic Origin (till, marine clay, alluvium, etc.)					RQD and correlation to rock mass quality (very poor, poor, etc.) ref: AASHTO Standard Specification for Highway Bridges			
Unified Soil Classification Designation					17th Ed. Table			
Groundwater level					Recovery Sample Container Labeling Requirements:			
Maine Department of Transportation					PIN		Blow Counts	
Geotechnical Section Key to Soil and Rock Descriptions and Terms Field Identification Information					Bridge Name / Boring Numbe		Sample Reco	overy
					Sample Numb	er	Personnel Ini	itials